TECHNICAL DATA SHEET

NO STRIP STOP END FORMWORK

PERFORMANCE OF NO STRIP STOP END FORMWORK

Tests have been carried out to verify the performance of No Strip Stop End Formwork at construction joints.

Assessment of shear, flexure and concrete strength at the joint confirms that the use of No Strip Stop End Formwork can improve the load achieved at this location.

Examination of cores taken at concreted No Strip Stop End Formwork joints shows full compaction around the No Strip Stop End Formworks and grout penetration at overlapping joints.



PROPERTIES OF NO STRIP STOP END FORMWORK

Section Modulus Z _{joint} Z _{span}	mm³/m mm³/m	952 1266	Rib in tension face in tension
Moment of Resistance (working) (fZ)	kNm/m kNm/m	0.184 0.244	At supports (rib away from load) At midspan (rib towards load)
Bending stiffness (EI) Working maximum reaction assumed max. working shear	kNm/m kNm/m kNm/m	2.00 10.88 5.44	See notes See notes See notes

NOTES

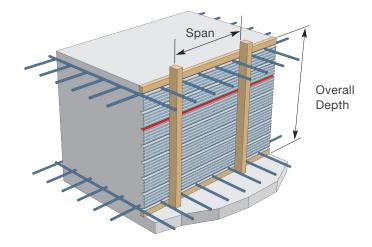
- 1. The properties assume that the No Strip Stop End Formwork is used with the ribs pointing into the concrete to be placed first and spanning in the strong direction between supports with the ribs parallel with the span.
- 2. The No Strip Stop End Formwork is considered a single use sacrificial material with a minimum factor of safety of 1.4 on ultimate failure. The failure stress being the minimum ultimate tensile strength of the No Strip Stop End Formwork rib sheet material.
- 3. The bending stiffness values should only be used for estimating deflections. They allow for the complex geometric changes in properties and shape as No Strip Stop End Formwork deflects.



SLAB CONSTRUCTION JOINTS: MAXIMUM SPACING ON TIMBER SUPPORTS



All temporary works must be designed by a competent Temporary Works Designer



INDICATIVE CLEAR DISTANCE BETWEEN SUPPORTS AT SLAB JOINTS

MAXIMUM CONCRETE PRESSURE

Overall depth of slab Joint	Theoretical* Table 2 CIRIA 108	Assumed for No Strip Stop End Formwork	Clear No Strip Stop End span between supports
mm	kN/m²	kN/m²	mm
250	6.25	3.20	950
500	12.50	6.30	675
750	18.75	9.50	550
1000	25.00	12.70	475
1250	31.25	15.80	425
1500	37.50	19.00	400
2000	50.00	25.30	350
2500	62.50	31.70	300
3000	75.00	38.00	275

NOTES

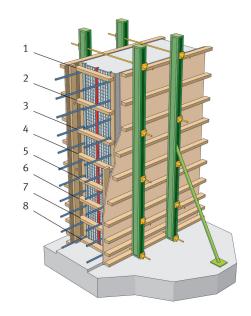
- 1. The No Strip Stop End Formwork is used in horizontal sheets with the ribs pointing into the concrete to be placed, and spanned in the strong direction between vertical supports.
- 2. The theoretical maximum pressure is that calculated using Table 2 in CIRIA report 108 with a concrete density of 25kN/m3, for EITHER a wall, base or column; AND applies to concrete groups 1 to 5 inclusive.



THIN WALL CONSTRUCTION JOINTS: MAXIMUM SPACING ON TIMBER SUPPORTS



All temporary works must be designed by a competent Temporary Works Designer



INDICATIVE SPACING OF SUPPORTS TO NO STRIP STOP END FORMWORK AT A VERTICAL JOINT IN A WALL

Position of the support measured from the top of the wall	Centre to centre spacing of the supports (mm) and the approximate height of wall (mm)			
	Support spacing	Approximate wall height		
1	550 500 450 400 375 325 275	See note 5 See note 5 See note 5 1500 1900 2275 2600 2875		
Load in the supports	Approximately 10.5 kN/m			

NOTES

- 1. The No Strip Stop End Formwork is used in vertical sheets with the ribs pointing into the concrete.
- 2. The value of the support spacing is measured centre to centre of the vertical supports and is NOT the clear distance.
- 3. The supports to the No Strip Stop End Formwork are horizontal and are a minimum of 50mm wide.
- 4. The approximate load per metre of the supporting members is given as a guide only.
- 5. It is assumed that the No Strip Stop End Formwork is continuous over at least three spans (i.e. over four horizontal supports). If this is not the case, refer to a designer for the increase in load on the supports.



CIRIA REPORT 108 - TEMPLATE ONLY

	Project:				Made By:	
	Location:				Checked By:	
	Client:				Date:	
	Job Title				Job.No/ Rev.	
					100,000	<u> </u>
NCRETE PRESSURE AN	ALYSIS:					REF.
1.0. Concrete Paramete	rs:					CIRIA Report 10
	(1 = Wall, 2 = Column), Concrete					
C1 = 1.00 √mh		e and shape of the f	ormwork			
Type = $\frac{T6}{C_2}$ unitless	,,	nctituant material	of concrete			
C2 = 0.60 V m	Coefficient dependent on the co	ristituent materials d	oj concrete			
$D = 25.00 \text{ kN/m}^3$	Weight Density of Concrete					
h = 3.30 m	Vertical Pour height					
H = 3.30 m	Vertical Form height		_			
R = 3.00 m/hr	The rate at which the concrete r		form			
T = 32.00 ° C	Concrete temperature at placing					
K = 0.56 unitles	(36/T+16)², Temperature Coeffi	CIEIIL				
2.0 CIRIA 108 - Provisio	ns					
- When C1√R >	H, the fluid pressure (Dh) should	l be taken as the de	esign pressure.			
P1 = 1.73 m	$C_{\overline{1}}\sqrt{R}$ incorporates the effects	of vibration and wor	·kahlitv			
P1 = 1.73 m H = 3.30 m	Vertical Form height	oj vioration ana wor	κασιιτή			
<u> </u>	_					
Pd1 = Press	ure is, D[C1 $\sqrt{R+C2K}\sqrt{H-C1}$	(R)])				
Pd1 = 53.87 kN/m ²	Calculated Pressure					
Pd2 = 82.50 kN/m ²	Dh, Pressure					
				CONCRETE PE		
- The Design Pressu	re is the smallest between the two	calculated values.		DIAGRA	<u>\M</u>	
D	esign Pressure = 53.86 kN/m²			3.5		
				0, 3.30		
Hz = 2.15 m	Height of Hydrostatic Zone			3		
				, \		
3.0 Values of Coefficien	ts C1. C2:			\		
J 2 1 2.2 22 31 232.1101011	/ -			2.5		
Walls: C1	1.00 : section where e	either the width or br	readth > 2.0 m	€ \		
Columns: C1	1.50 : section where b	ooth the width and b	readth < 2.0 m	VERTICAL POUR HEIGHT (f)		
Concrete		Value of C2		포 2		
	PC without admixtures	0.30) j		
	PC without admixtures,	0.30		Ĭ 1 - \		
except a retarder			_	<u> </u>		
- OPC, RHPC or SR		0.45	Р	ERT	53.87,	
	PFAC or blends containing	0.45	- -	<u>₹</u> - >	1.145	
, 55	rfs or 40% pfa without]	T			
admixtures	PFAC or blends containing	0.45				
	of sor 40% pfa with any	0.45	H/3	0.5		
admixtures, exce]	-			
	PFAC or blends containing	0.60			50.07	
	rfs or 40% pfa with a			o	53.87, 0.00	
retarder				0 5	0.00	
	more than 70% ggbfs or	0.60		CONCRETE PRE	SSURE (Pd)	
40% pfa		1			* · · /	1

